

BOOK =2

**TELEDYNE
NATIONAL TRACING PAPER

Procedures

A. Determining Available Sight Distance.

The available sight distance (SD) is determined by making observations at each site under study. Assumptions and criteria used for this part of the study are as follows.

- 1. The driver's eye height is 3.50 feet.
- The height of the target used at the driveway where passengers are picked up or discharged is 4.0 feet. This is approximately 1/2 the height of a school bus and would include the flashing lights and a substantial portion of the bus itself.
- The observation point is positioned on the roadway at approximately the same lateral location as the driver's eye.

By using the above assumptions and criteria, the available sight distance is determined.

B. Determining The Stopping Sight Distance.

The method of computing stopping sight distance is taken from the 1984 edition of A Policy on Geometric Design of Highways and Streets published by AASHTO. Stopping sight distance is composed of two parts. First is the distance traveled during perception and reaction and second is the actual distance to brake to a stop. The following formula is used.

$$d = V(1.47)(t) + -\frac{V^2}{30(f+9)}$$

The terms in this equation are defined as follows.

- d = The total stopping sight distance in feet.
- V = The initial speed in miles per hour.
- t = The perception/reaction time in seconds.
- f = The coefficient of friction between the tires and the roadway.
- g = The grade expressed as a decimal.

Assumptions made in computing the stopping sight distance are as follows.

- The initial speed is 60 MPH for a rural location which allows for a factor of safety over the posted 55 MPH speed limit. If the location is in a reduced speed zone, use the actual posted speed limit.
- Perception time is 1.5 seconds.
- 3. Reaction time is 1.0 seconds.
- 4. Total perception reaction time (t) equals 2.5 seconds.
- The coefficient of friction (f) is 0.30 which is representative of wet povement conditions.
- The grade used is the actual percent divided by 100 or the average grade if the approach is on a vertical curve.

BORMULAE FOR SOLVING RIGHT TRIANGLES

IM 9.101

The following is an example showing how the stopping sight distance should be computed. Data to be used is as follows.

t = 2.5 seconds

f = 0.300 (wet pavement)

g = -4.50% or 4.50/100 = 0.045

$$d = V(1.47)(t) + \frac{V^2}{30(f+g)}$$

$$d = 60 (1.47)(2.5) + \frac{(60)^2}{30(0.300 - 0.045)}$$

$$d = 220.5 + \frac{3600}{30(0.255)} = 2.20.5 + \frac{120}{0.3\pm0}$$

$$d = 220.5 + 470.6$$

d = 691.1, rounded to 690 ft.

C. Determination of Signing Needs.

The available sight distance (SD) needs to be compared with the stopping sight distance (d) to determine if a School Bus Stop Ahead sign is needed.

When the situation involves approaching the front of a stopped school bus, 10 feet should be added for a passenger crosswalk and 25 feet for a clear zone. The analysis would be as follows.

- (1a) If SD is greater than d + 10 + 25, a School Bus Stop Ahead sign is not justified.
- (1b) If SD is equal to or less than d + 10 + 25, a School Bus Stop Ahead sign is justified.

When the situation involves approaching the rear of a stopped school bus, 35 feet follows.

The analysis is as

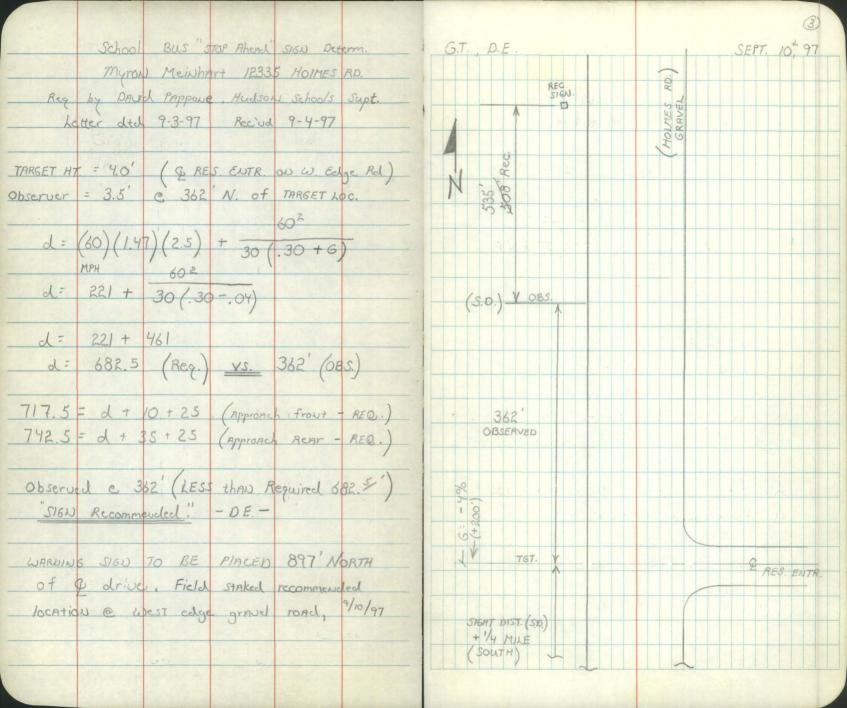
- (2a) If SD is greater than d + 35 + 25, a School Bus Stop Ahead sign is
- (2b) If SD is equal to or less than d + 35 + 25, a School Bus Stop Ahead sign is justified.

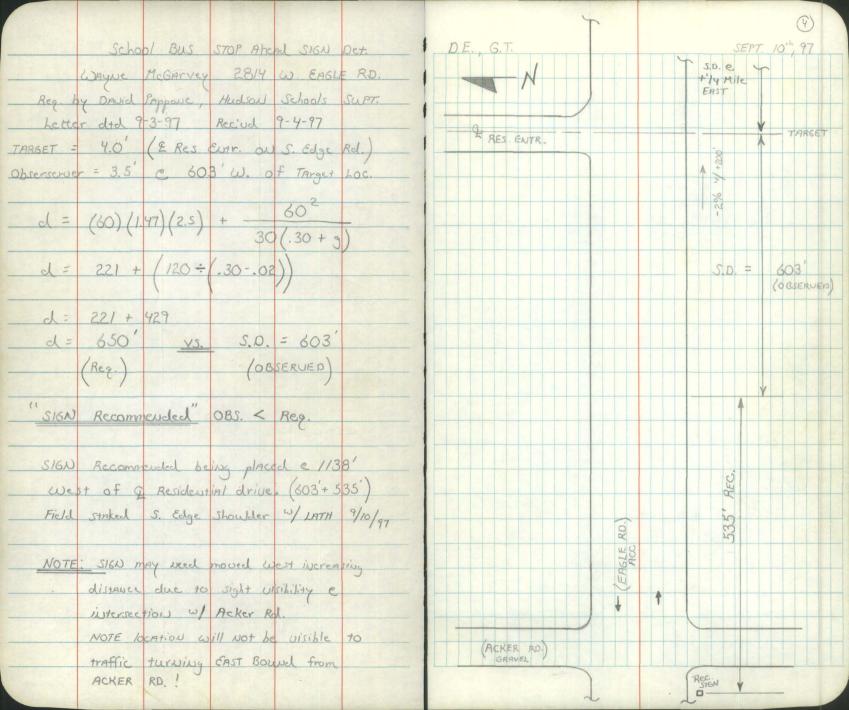
D. Installation of a Sign.

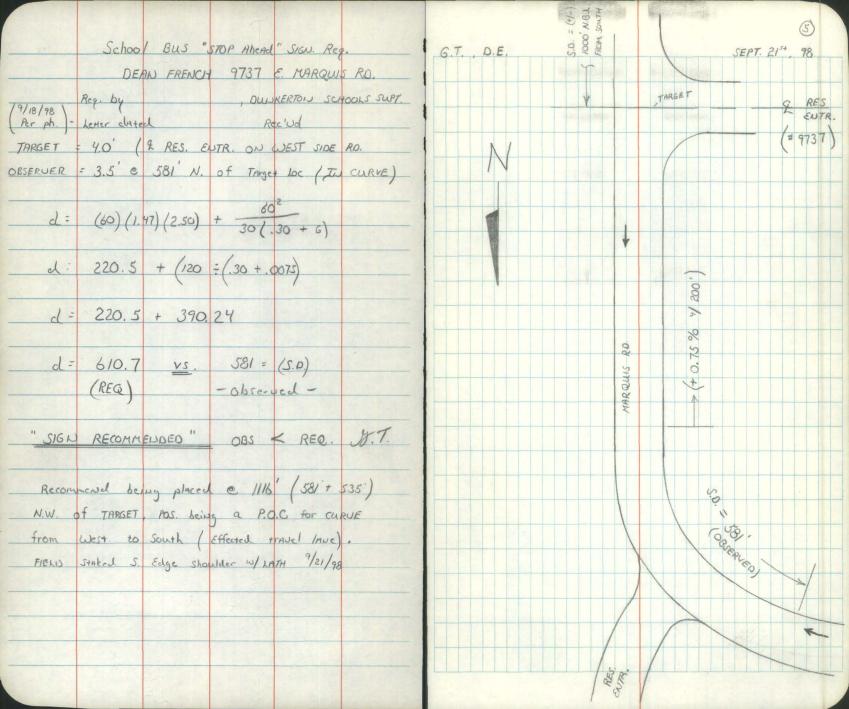
If a School Bus Stop Ahead sign is justified under 1b or 2b in Section C above, a standard 30" x 30" warning sign should be used. It should be installed approximately 500 feet in advance of the point where 1/2 of the school bus is

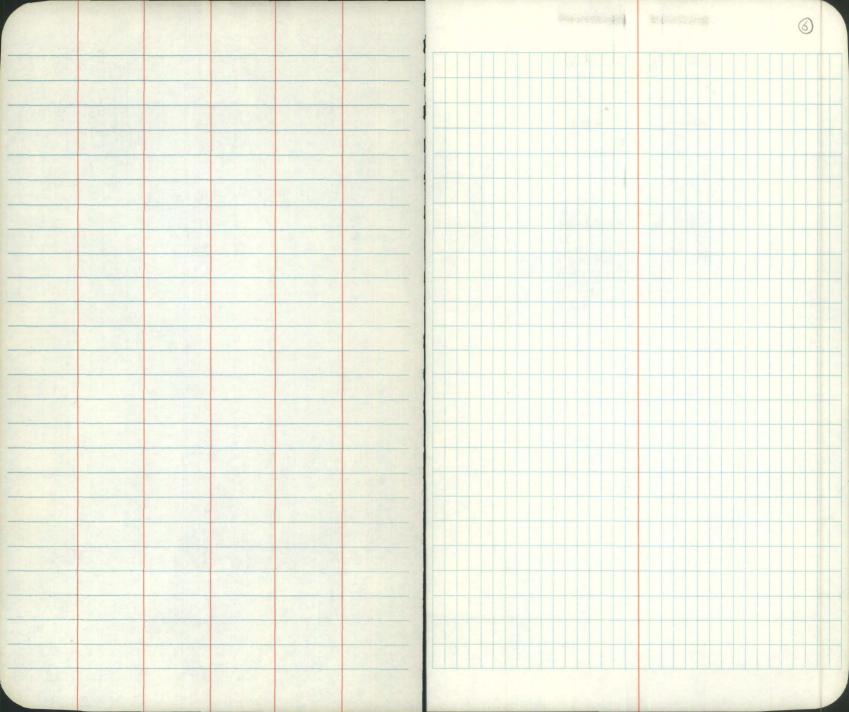
	The state of the s	LNUEN
School Bus S	Stap Ahead	
	or Sign Placeme	ent
	Calculations	
311 21311 7	Dece of vois	
Resident Ad	(ress	0
	E. Dunk, Rd.	1998
		2
	HOITES RA	J 4
	w. Engle Rd	7
DEAN FRENCH		5

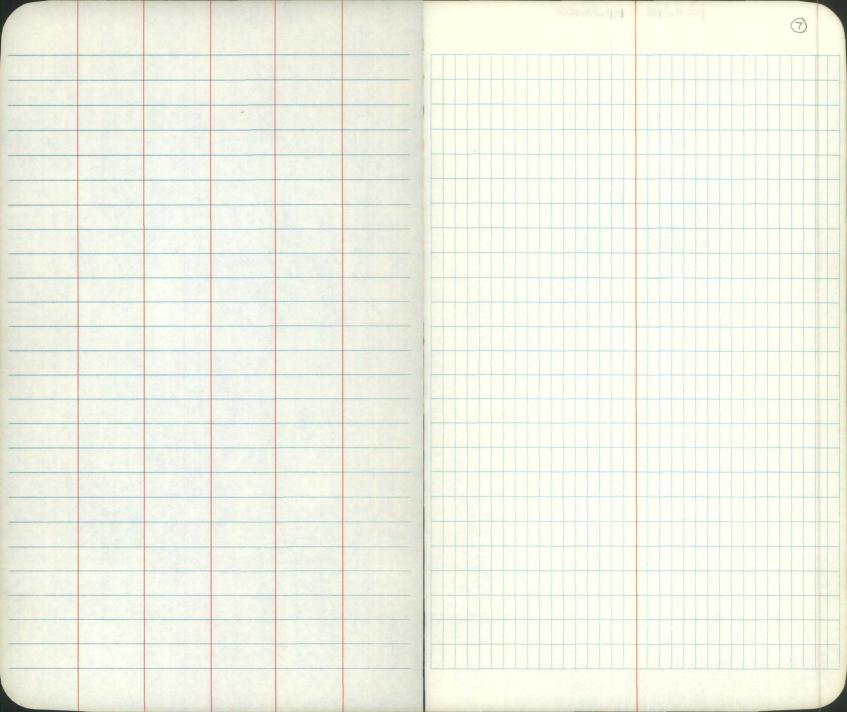
3 of 3

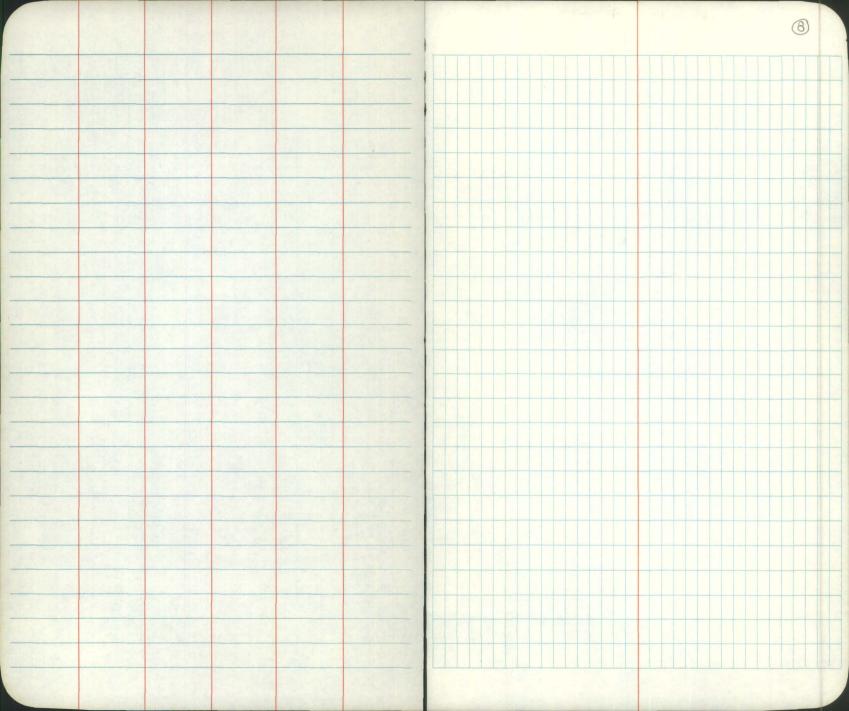


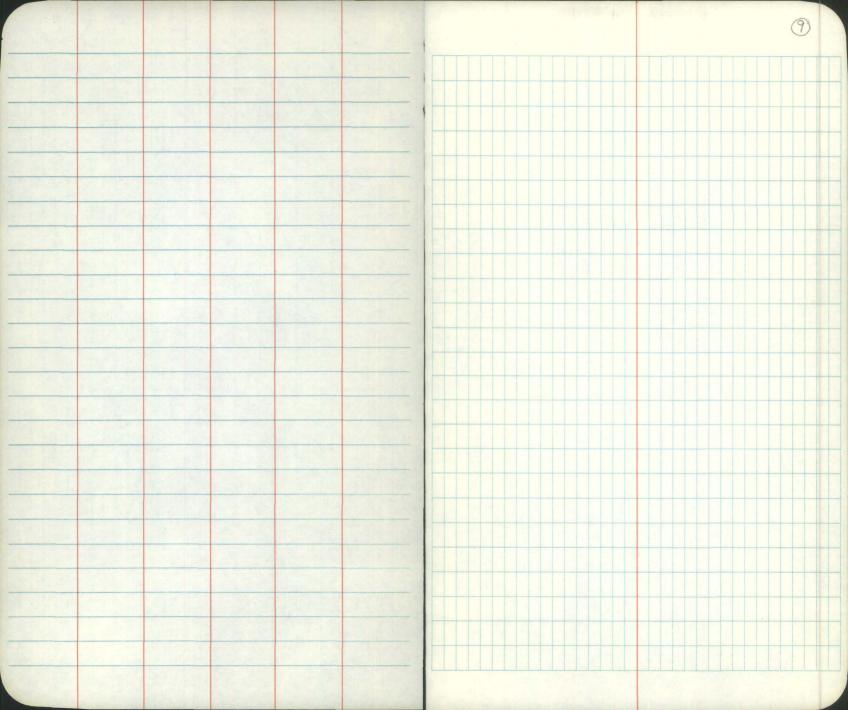


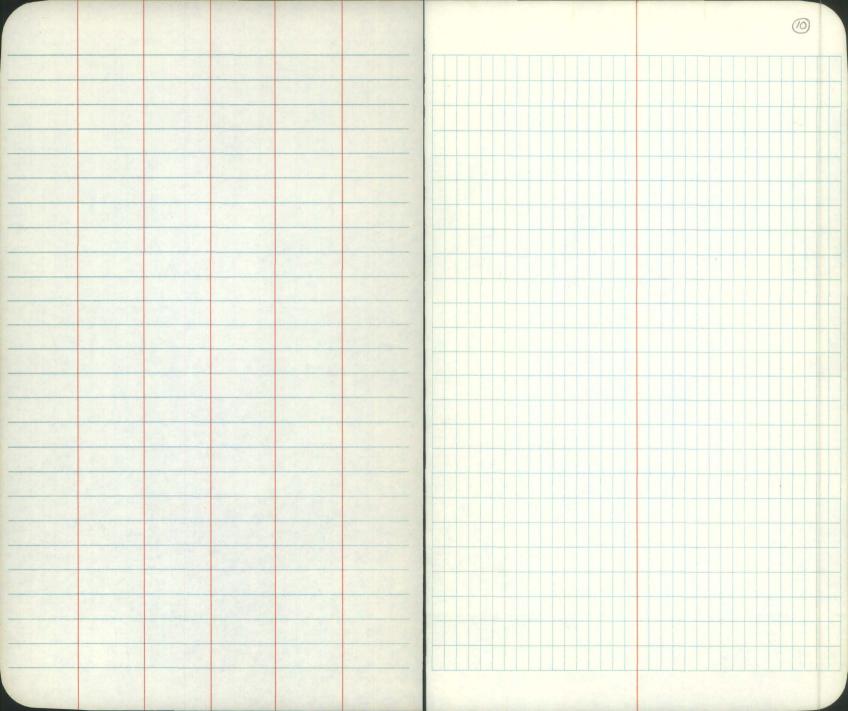


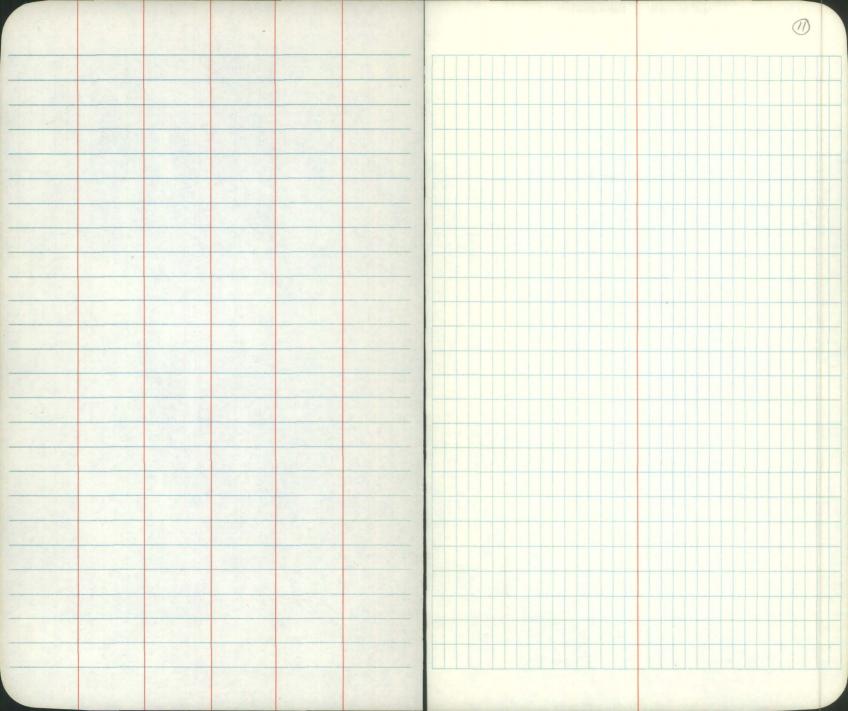


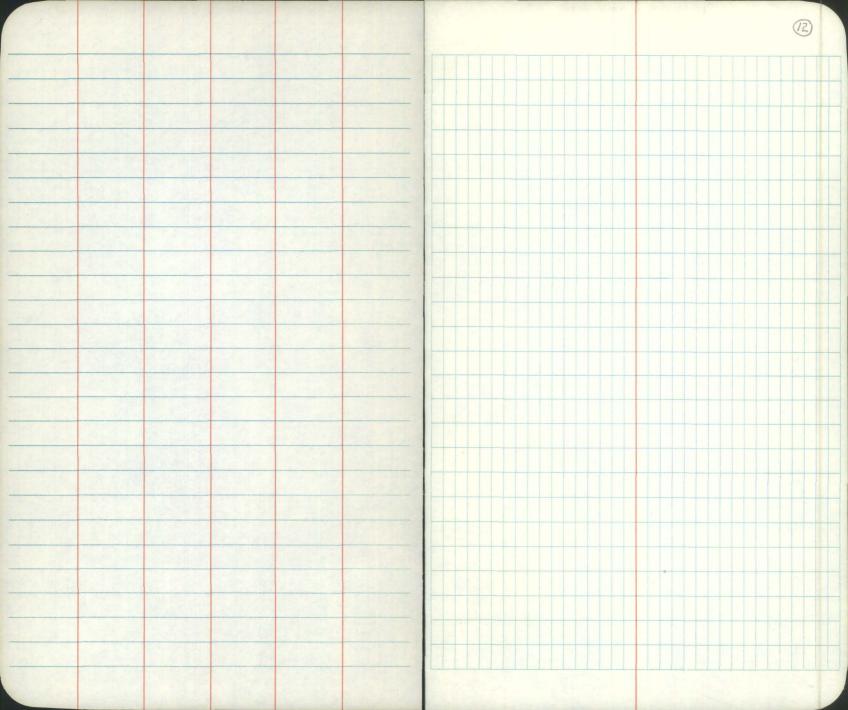


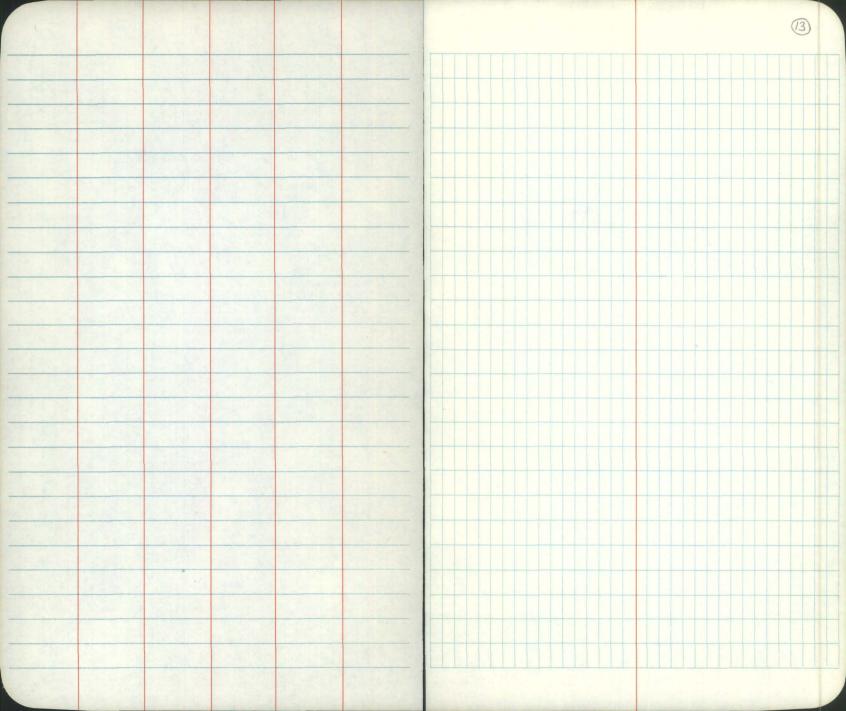


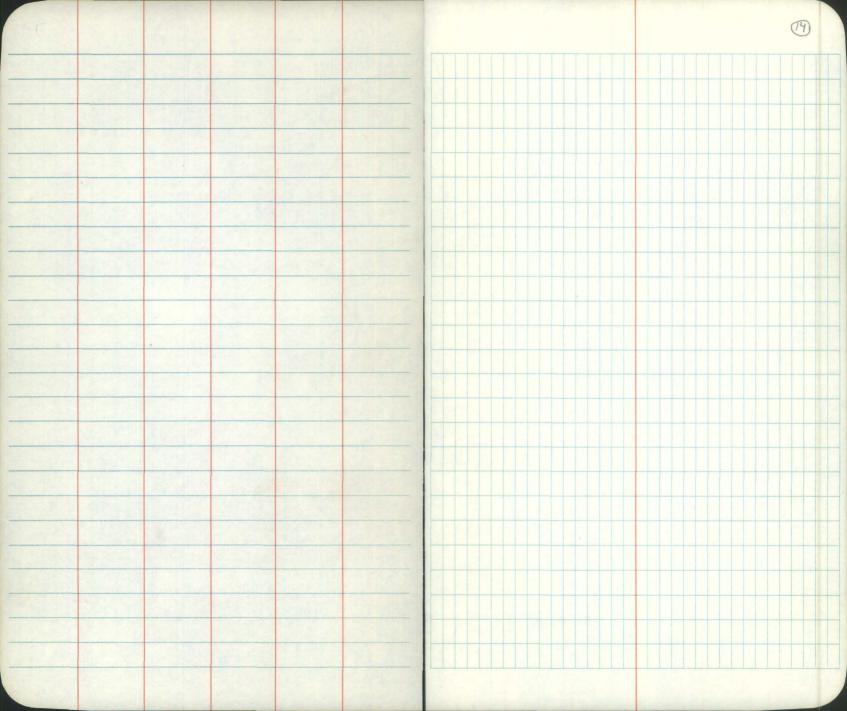


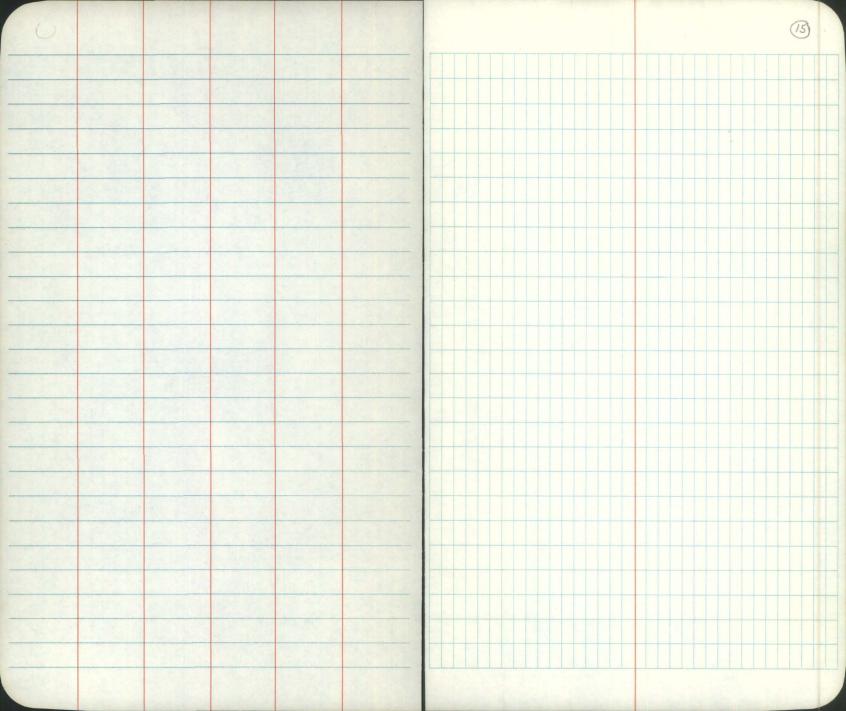


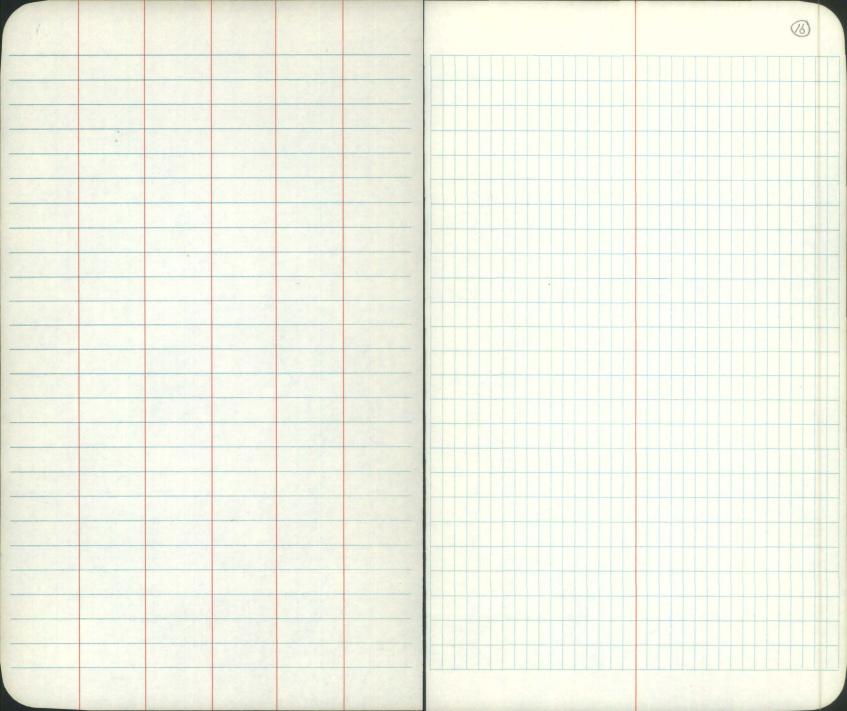


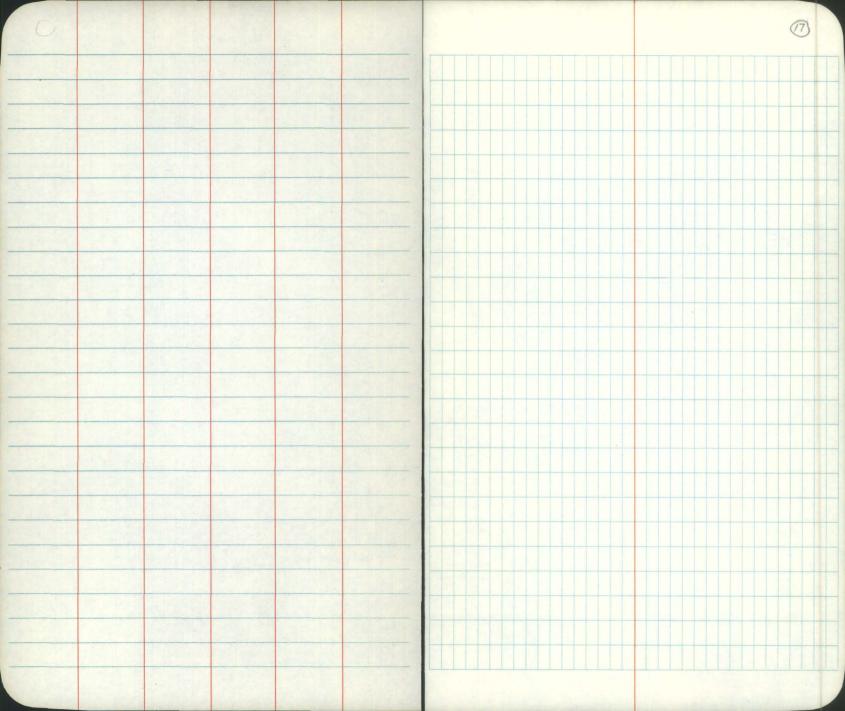


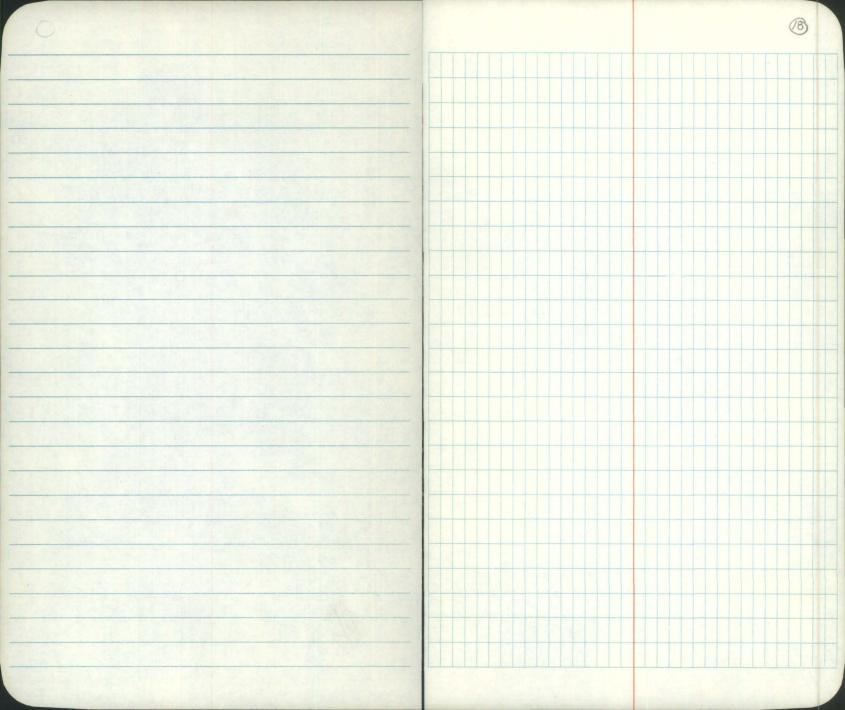


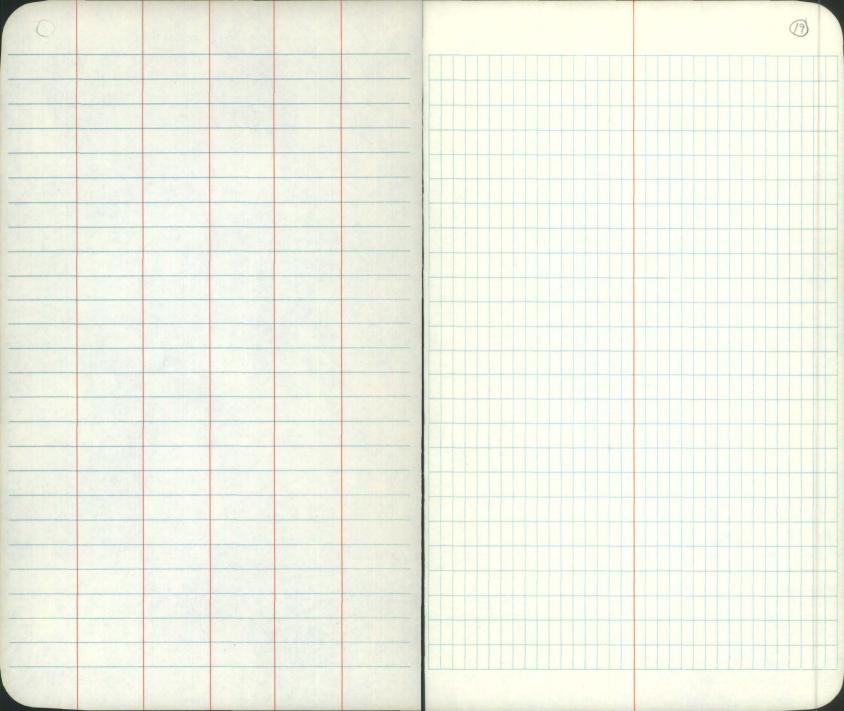


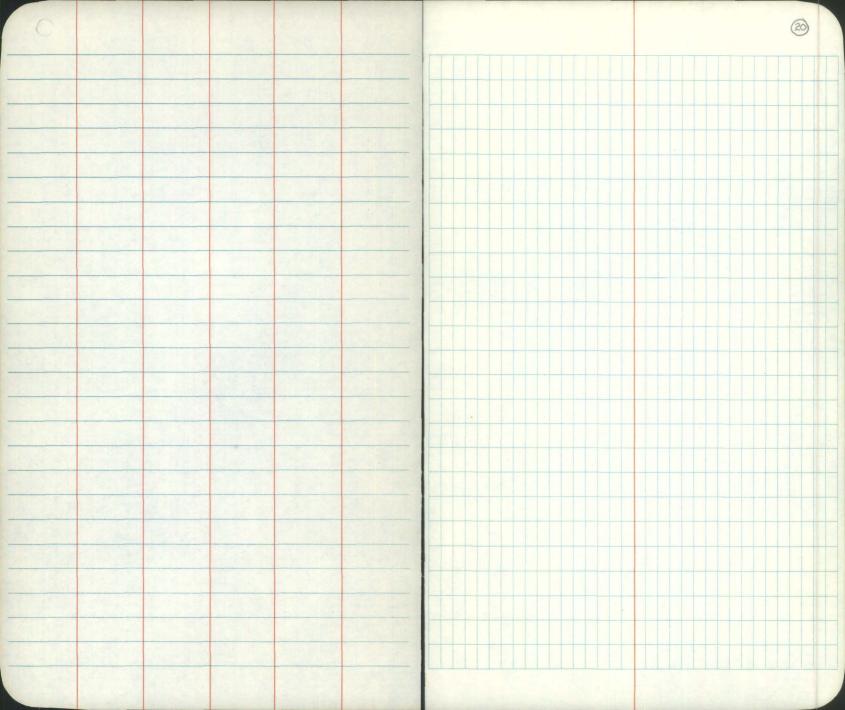


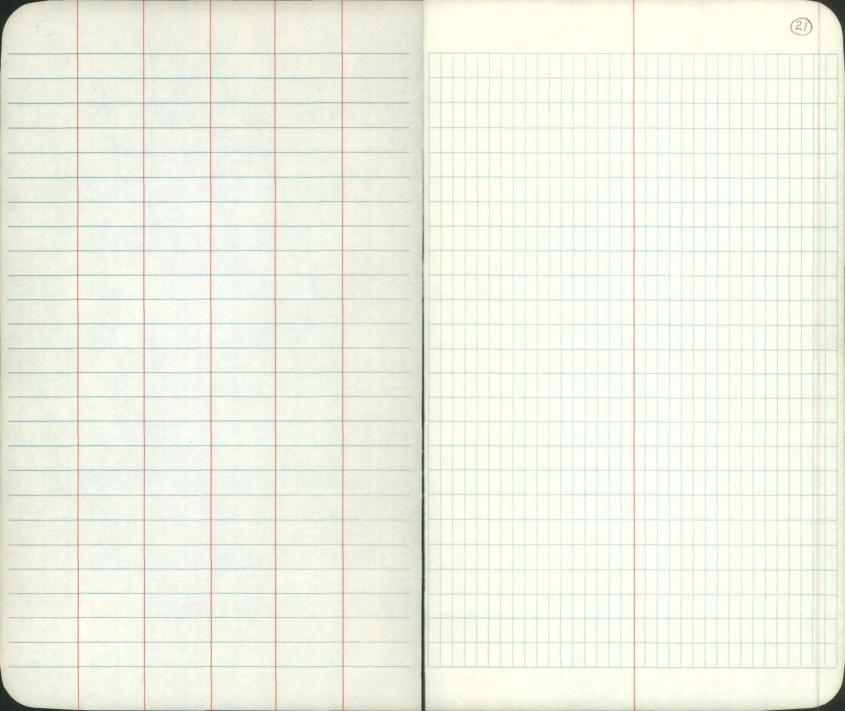


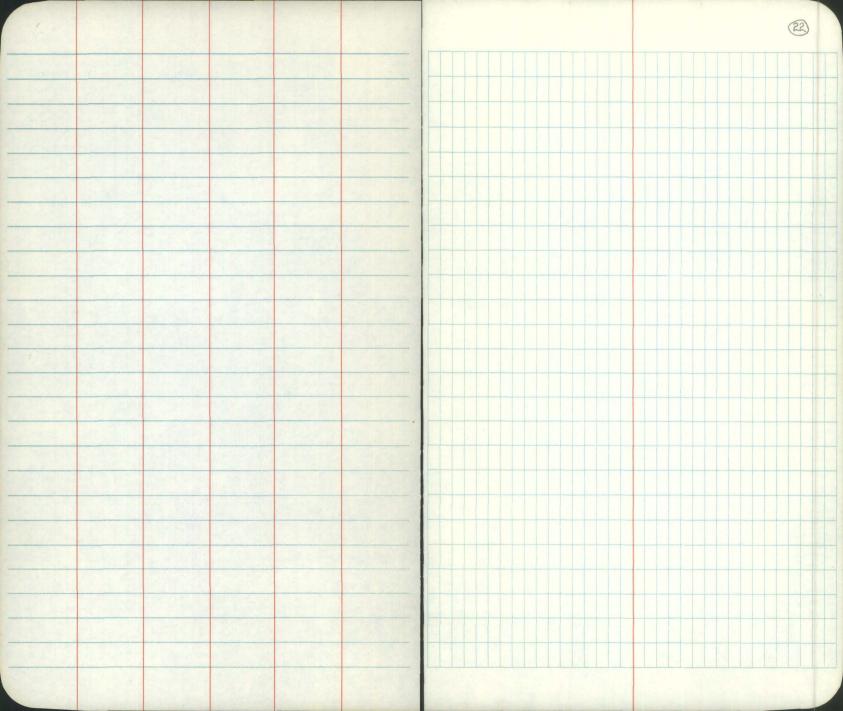


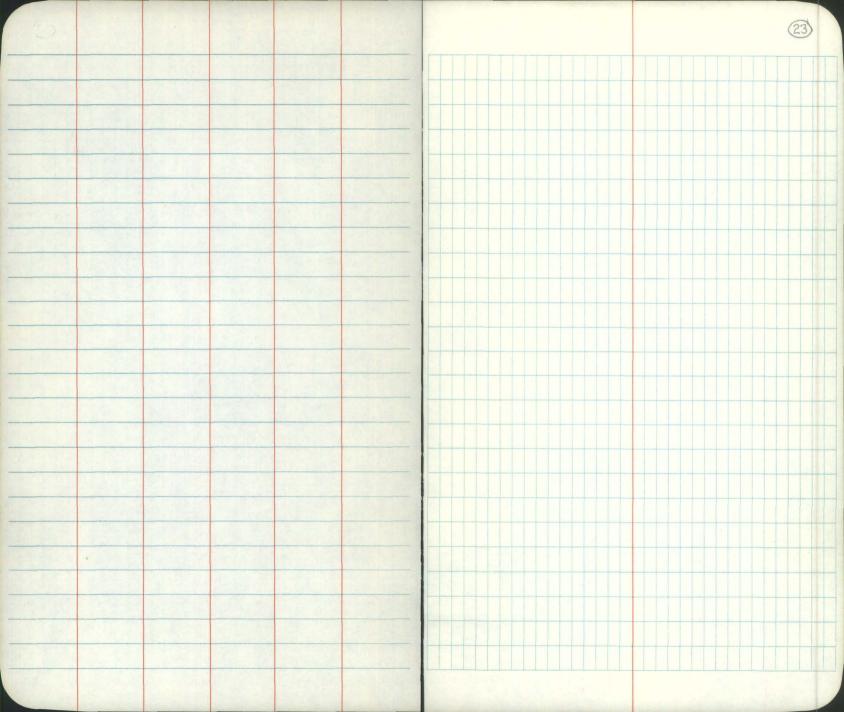


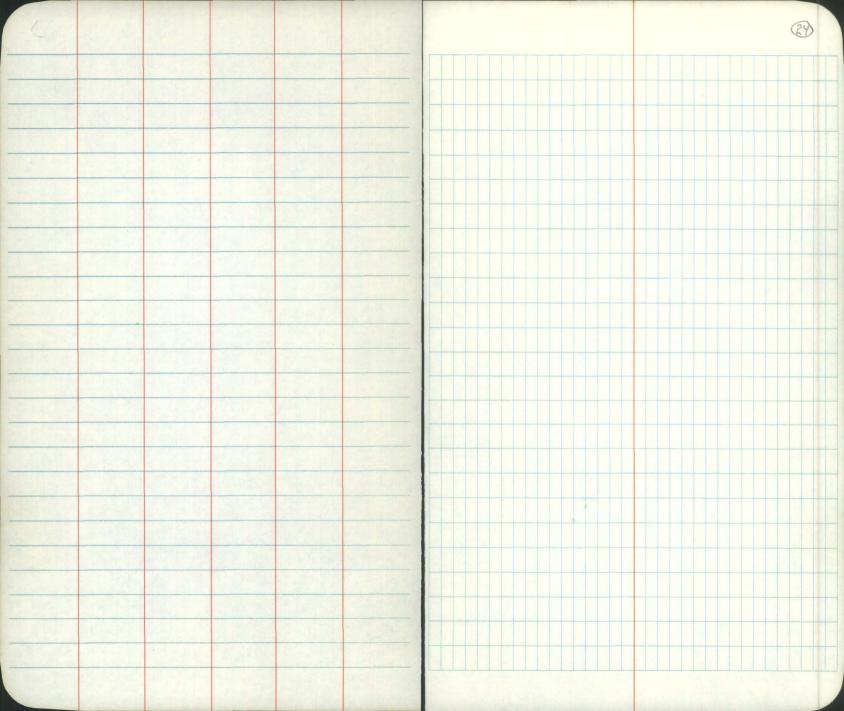


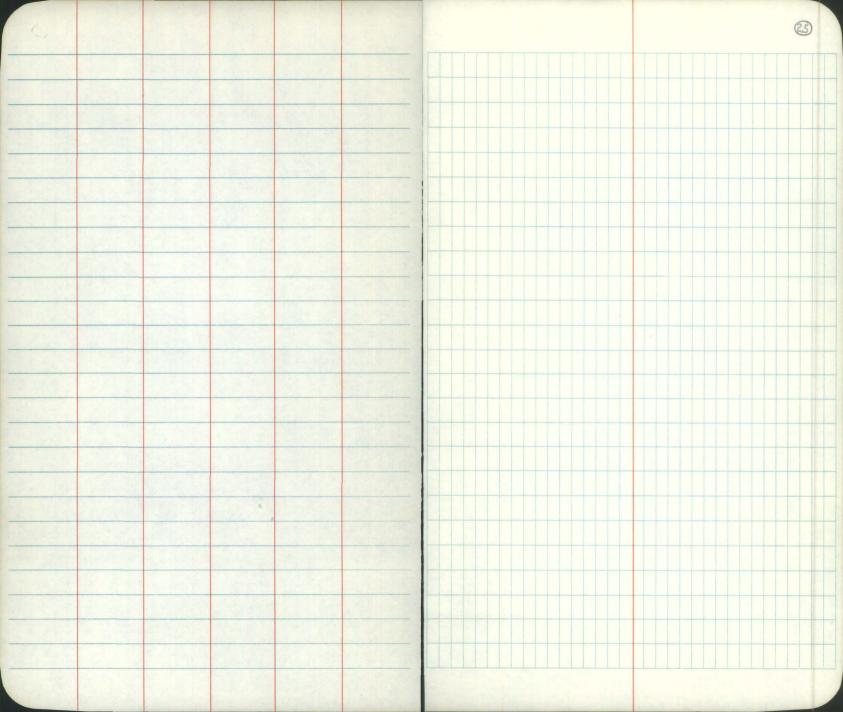


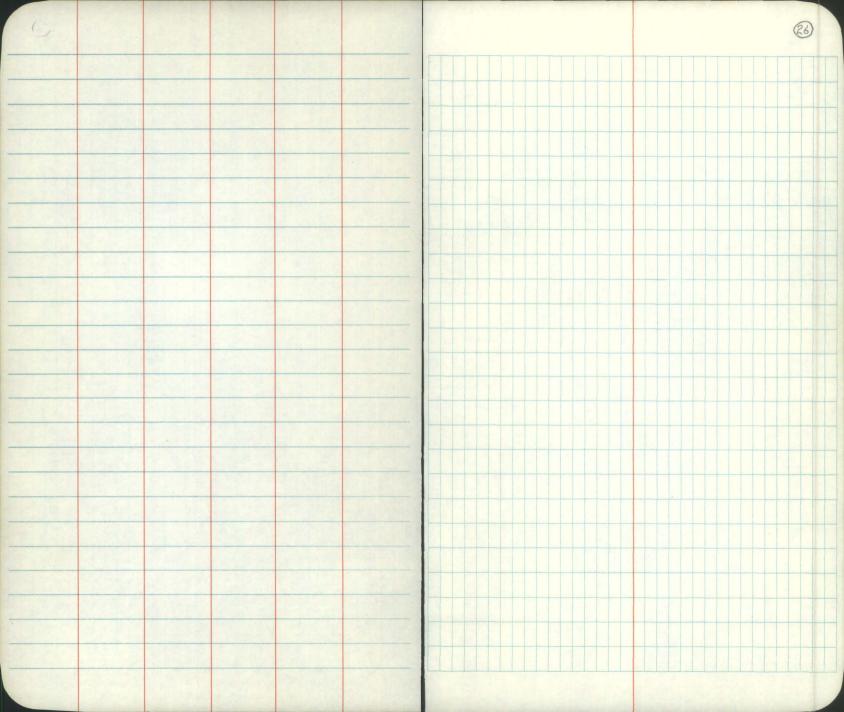


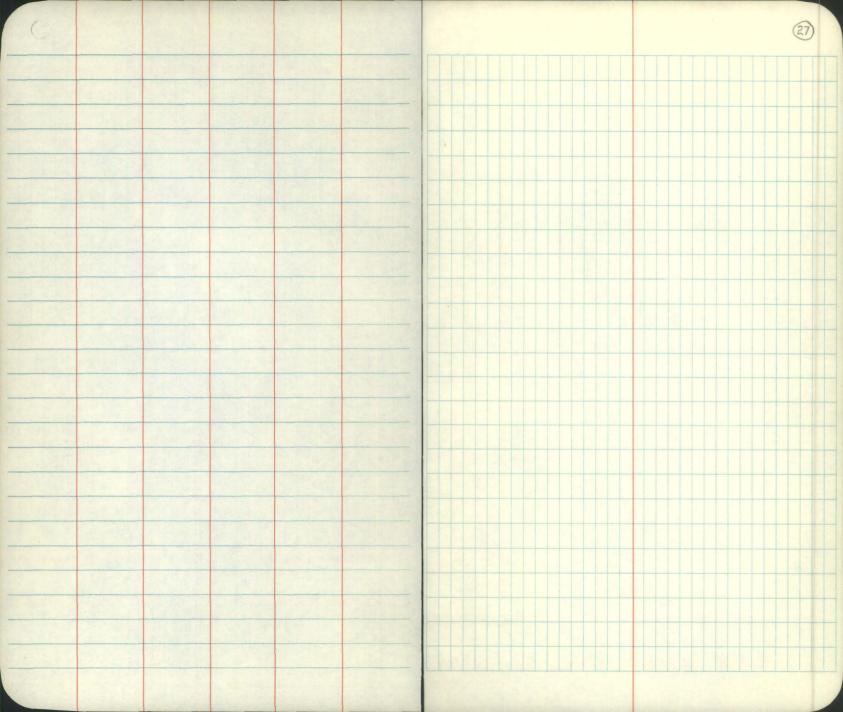


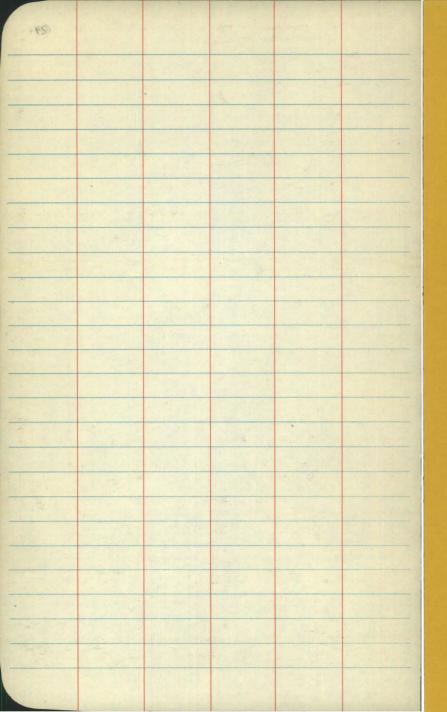












CURVE FORMULAE

D=Degree of Curve

1°=1-Degree of Curve

2°=2-Degree of Curve

P.C.=Point of Curve

P.T.=Point of Tangent

P.I.=Point of Intersection

I=Intersection of Angle, Angle

Between Two Tangents

L=Length of Curve,

from P.C. to P.T.

T=Tangent Distance

E=External Distance

R=Radius

L.C.=Length of Chord

M := Length of Middle Ordinate

c=Length of Sub-Chord d=Angle of Sub-Chord

$$R = \frac{L.C.}{2 \sin \frac{1}{2} I}$$
 $T = R \tan \frac{1}{2} I = \frac{L.C.}{2 \cos \frac{1}{2} I}$

$$\frac{\text{L.C.}}{2}$$
 = R sin $\frac{1}{2}$, D 1° = R = 5730, D 2° = $\frac{5730}{2}$, D = $\frac{5730}{R}$

$$M = R (1 - \cos \frac{1}{2} I), = R - R \cos \frac{I}{2}$$

$$\frac{E+R}{R} = \sec \frac{I}{2}, \frac{R-M}{R} = \cos \frac{I}{2}$$

$$c=2R\sin{\frac{1}{2}}d, d=\frac{c}{2R}$$

L.C. =
$$2 R \sin \frac{1}{2} I$$
, $E = R (\sec \frac{1}{2} I - 1)$, $= R \sec \frac{I}{2} - R$

Minutes in Decimals of a Degree.

1'	.0167	11'	.1833	21'	.3500	31'	.5167	41'	.6833	51'	.8500
2	.0333	12	.2000	22	.3667	32	.5333	42	.7000	52	.8657
3	.0500	13	.2167	23	.3833	33	.5500	43	.7167	53	.8833
4	.0667	14	.2333	24	.4000	34	.5667	44	.7333	54	.9000
5	.0833	15	.2500	25	.4167	35	.5833	45	.7500	55	.9167
6	.1000	16	.2667	26	.4333	36	.6000	46	.7667	56	.9333
7	.1167	17	.2833	27	.4500	37	.6167	47	.7833	57	.9500
8	.1333	18	.8000	28	.4667	38	.6333	48	.8000	58	.9667
9	.1500	19	.3167	29	.4833	39	.6500	49	.8167	59	.9833
10	.1667	20	.3333	30	.5000	40	.6667	50	.8333	60	1.0000

Inches in Decimals of a Foot.

⅓ ₆ .0052	352 .0078	.0104	₹6 .0156	.0208	%6 .0260	3/8 .0313	½ .0417	5⁄8 .0521	3/4 .0625	% .0729
.0823	2 .1667	3 .2500	.3333	5 .4167	.5000	7 .5833	.6667	9 .7500	10 .8333	.9167